

## 1. GENERAL NOTES

- 1.1. THE GOVERNING CODE FOR THIS PROJECT IS THE FLORIDA BUILDING CODE, 5th EDITION (2014). THIS CODE PRESCRIBES WHICH EDITION OF EACH REFERENCE STANDARD APPLIES TO THIS PROJECT.
- 1.2. TO THE BEST OF OUR KNOWLEDGE, THE STRUCTURAL DRAWINGS COMPLY WITH THE APPLICABLE REQUIREMENTS OF THE GOVERNING BUILDING CODE.
- 1.3. CONSTRUCTION IS TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING BUILDING CODE AND ALL OTHER APPLICABLE FEDERAL, STATE, AND LOCAL CODES, STANDARDS, REGULATIONS AND LAWS.
- 1.4. THE STRUCTURAL DOCUMENTS ARE TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL & CIVIL DOCUMENTS. IF A CONFLICT EXISTS, THE MORE STRINGENT GOVERNS.
- 1.5. DETAILS LABELED "TYPICAL" APPLY TO ALL SITUATIONS THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED, WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION. QUESTIONS REGARDING THE APPLICABILITY OF TYPICAL DETAILS SHALL BE RESOLVED BY THE ARCHITECT.
- 1.6. CONTRACTORS WHO DISCOVER DISCREPANCIES, OMISSIONS OR VARIATIONS IN THE CONTRACT DOCUMENTS DURING BIDDING SHALL IMMEDIATELY NOTIFY THE ARCHITECT. THE ARCHITECT WILL RESOLVE THE CONDITION AND ISSUE A WRITTEN CLARIFICATION.
- 1.7. THE CONTRACTOR SHALL PROTECT ADJACENT PROPERTY, HIS OWN WORK AND THE PUBLIC FROM HARM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS AND METHODS, AND JOBSITE SAFETY INCLUDING ALL OSHA REQUIREMENTS.
- 1.8. THE STRUCTURE IS DESIGNED TO BE STRUCTURALLY SOUND WHEN COMPLETED. PRIOR TO COMPLETION, THE CONTRACTOR IS RESPONSIBLE FOR STABILITY AND TEMPORARY BRACING.

## 2. RECOMMENDED SEQUENCE OF CONSTRUCTION

- 2.1. REMOVE EXISTING SIDEWALK WHERE NECESSARY AND EXCAVATE AROUND EXTERIOR OF NORTHEAST CORNER OF EXISTING STRUCTURE.
- 2.2. INSTALL HELICAL PILES AT CONTINUOUS BUILDING FOOTING. THESE PILES SHALL BE INSTALLED TO STABILIZE THE EXISTING STRUCTURE WITH NO LIFTING OF THE EXISTING STRUCTURE REQUIRED.
- 2.3. PERFORM PHASES 1 AND 2 FROM SHEET S3.2 FOR THE NORTHEAST STAIR (AND THE NORTHWEST STAIR AT THE DISCRETION OF THE CONTRACTOR).
- 2.4. INSTALL HELICAL PILES TO SUPPORT THE EXTERIOR STAIR WALL FOUNDATION AND LIFT INCREMENTALLY TO RETURN WALL TO PLUMB. TEMPORARY SUPPORT OF THE STAIR STRUCTURE MAY BE REQUIRED DURING THE STAIR FOUNDATION STABILIZATION PROCESS.
- 2.5. PERFORM STEEL REPAIRS ON SHEET S3.1, PHASES 3 AND 4 ON SHEET S3.2, AND ON SHEET 3.3. FOR NORTHEAST AND NORTHWEST EXTERIOR STAIRS.
- 2.6. INJECT THE SOILS BENEATH THE INTERIOR LOAD BEARING WALL AND THE STAIR WALL FOUNDATION WITH URETHANE FOAMING GROUT.
- 2.7. BACKFILL ALL EXCAVATIONS AND REPLACE SIDEWALK IN KIND TO RETURN THE SIDEWALK AND SITE TO PRE-CONSTRUCTION CONDITION. REFER TO AS-BUILT PLANS FOR CONSTRUCTION REQUIREMENTS RELATED TO THE EXTERIOR CONCRETE.

## 3. DESIGN LOADS

- 3.1. GENERAL DESIGN LOADS FOR THE ROOM OCCUPANCIES ARE LISTED BELOW. FOR A FULL LIST OF THE DESIGN LOADS ON THE STRUCTURE, SEE THE PREVIOUSLY SUBMITTED STRUCTURAL ANALYSIS OF THE EXISTING STRUCTURE.
- 3.2. ROOF LOAD = 20 PSF LIVE / 15 PSF DEAD
- 3.3. OFFICE LOAD = 40 PSF LIVE / 20 PSF DEAD
- 3.4. CORRIDOR LOAD = 80 PSF LIVE / 20 PSF DEAD
- 3.5. ASSEMBLY LOAD = 100 PSF LIVE / 20 PSF DEAD

## 3.6. WIND LOAD CRITERIA (PER ASCE 7)

- 3.6.1. WIND SPEED (ULT/ASD) = 120 MPH / 93 MPH
- 3.6.2. RISK CATEGORY = II
- 3.6.3. WIND EXPOSURE CATEGORY = B
- 3.6.4. ENCLOSURE CLASSIFICATION = ENCLOSED

## 4. HELICAL PILE AND GEOTECHNICAL INFORMATION

- 4.1. SEE SHEETS S0.3 AND S0.4 FOR GEOTECHNICAL INFORMATION IN THE VICINITY OF THE PROPOSED WORK.
- 4.2. SEE SHEET S0.2 FOR HELICAL PILE DESIGN INFORMATION.
- 4.3. THE FOLLOWING MATERIALS WERE USED AS A BASIS OF DESIGN FOR THE STAIR AND BEARING WALL SUPPORT SHOWN. ALTERNATE HELICAL PILE COMPONENTS EQUIVALENT TO THOSE SHOWN BELOW MAY BE CONSIDERED PROVIDED DOCUMENTATION CONTESTING TO THE STRUCTURAL EQUIVALENCY IS SUBMITTED FOR REVIEW:
  - 4.3.1. HELICAL PILE SHAFT = CHANCE SS5
  - 4.3.2. FOOTING CONNECTION BRACKET = CHANCE C1500121
  - 4.3.3. HELICAL PILE HELICES = PER PILE MANUFACTURER TO OBTAIN CAPACITIES SHOWN

## 5. STRUCTURAL STEEL

- 5.1. FABRICATE AND ERECT STRUCTURAL STEEL IN CONFORMANCE, W/ AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", WITH COMMENTARY, AND ALL OSHA REQUIREMENTS.
- 5.2. STRUCTURAL STEEL SHAPES SHALL BE FABRICATED FROM THE FOLLOWING MATERIALS:
  - PLATES AND BARS: ASTM A36,  $f_y=36$  ksi.
  - COLD-FORMED HOLLOW STRUCTURAL SECTIONS (HSS):
    - SQUARE AND RECTANGULAR SECTIONS: ASTM A500, GRADE B,  $f_y=46$  KSI.
- 5.3. ALL SHOP AND FIELD WELDING SHALL CONFORM TO THE AWS D1.1 STRUCTURAL WELDING CODE BY THE AMERICAN WELDING SOCIETY. USE E70 SERIES WELDING ELECTRODES, U.O.N. WHERE NECESSARY, REMOVE GALVANIZING OR PRIMER PRIOR TO WELDING.
- 5.4. BOLTS SHALL COMPLY WITH "SPECIFICATION FOR STRUCTURAL JOINTS USING: ASTM A325 OR A490 BOLTS", INCLUDING COMMENTARY.
- 5.5. SETTING BASE AND BEARING PLATES:
  - 5.5.1. CLEAN CONCRETE BEARING SURFACE OF BOND-REDUCING MATERIALS AND CLEAN BOTTOM OF BASE PLATE.
  - 5.5.2. SET BASE OR BEARING PLATE ON WEDGE OR OTHER ADJUSTING DEVICES AS REQUIRED.
  - 5.5.3. TIGHTEN ANCHOR RODS AFTER STRUCTURAL STEEL FRAME HAS BEEN PLUMBED. DO NOT REMOVE WEDGES OR SHIMS BUT, IF PROTRUDING, CUT OFF FLUSH WITH EDGE OF BASE OR BEARING PLATE PRIOR TO PACKING WITH GROUT.
  - 5.5.4. PACK OR POUR NON-SHRINK GROUT SOLIDLY BETWEEN BEARING SURFACE AND BASE OR BEARING PLATE. ENSURE THAT NO VOIDS REMAIN. FINISH EXPOSED SURFACES, PROTECT GROUT AND ALLOW TO CURE.
  - 5.5.5. FOR PROPRIETARY GROUT MATERIALS, COMPLY WITH MANUFACTURER'S INSTRUCTIONS.
  - 5.5.6. BASE PLATES MUST BE GROUTED A MINIMUM OF 72 HOURS PRIOR TO PLACING CONCRETE SLABS ON SUPPORTING STEEL STRUCTURE.
- 8.8. CUT, DRILL, OR PUNCH HOLES PERPENDICULAR TO METAL SURFACES. REAM HOLES THAT MUST BE ENLARGED TO ADMIT BOLTS AS PERMITTED BY ARCHITECT. DO NOT ENLARGE UNFAIR HOLES BY BURNING OR USING DRIFT PINS.

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GENERAL NOTES

S0.1

## HELICAL PILE DESIGN INFORMATION

CHANCE ANCHORS ASSEMBLY = SS5 LEAD & EXTENSION  
 SHAFT SIZE = 1.50" SOLID SQUARE BAR  
 TORQUE CORRELATION FACTOR = 10 FT<sup>-1</sup>  
 COUPLING BOLTS = (1) 5/8"Ø A325 TYPE 1 HEX HEAD BOLT WITH  
 THREADS EXCLUDED FROM SHEAR PLANES  
 MAXIMUM ULTIMATE SHAFT CAPACITY = 70,000 LB (AT MAX TORQUE)  
 HELIX CONFIGURATION = TO BE DETERMINED BASED MINIMUM EMBEDMENT DEPTHS  
 AND INSTALLATION TORQUES EXISTING CONDITIONS. MINIMUM  
 TWO HELICES REQUIRED.

MINIMUM INSTALLATION TORQUE =  $\frac{\text{REQUIRED ULTIMATE STRENGTH}}{\text{TORQUE CORRELATION FACTOR}}$

MAXIMUM INSTALLATION TORQUE = 5,700 LBS\*FT

MINIMUM EMBEDMENT DEPTH = 15'-0"

<u>ALLOWABLE DESIGN CAPACITY</u>	<u>ULTIMATE DESIGN CAPACITY</u>	<u>TORQUE CORRECTION FACTOR</u>	<u>MINIMUM INSTALLATION TORQUE</u>
15,000 LBS.	30,000 LBS.	10 FT <sup>-1</sup>	3,000 LBS.*FT
12,000 LBS.	24,000 LBS.	10 FT <sup>-1</sup>	2,400 LBS.*FT
10,000 LBS.	20,000 LBS.	10 FT <sup>-1</sup>	2,000 LBS.*FT
9,000 LBS.	18,000 LBS.	10 FT <sup>-1</sup>	1,800 LBS.*FT

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HELICAL PILE  
 DESIGN  
 INFORMATION

S0.2

**FIGURE LEGEND**

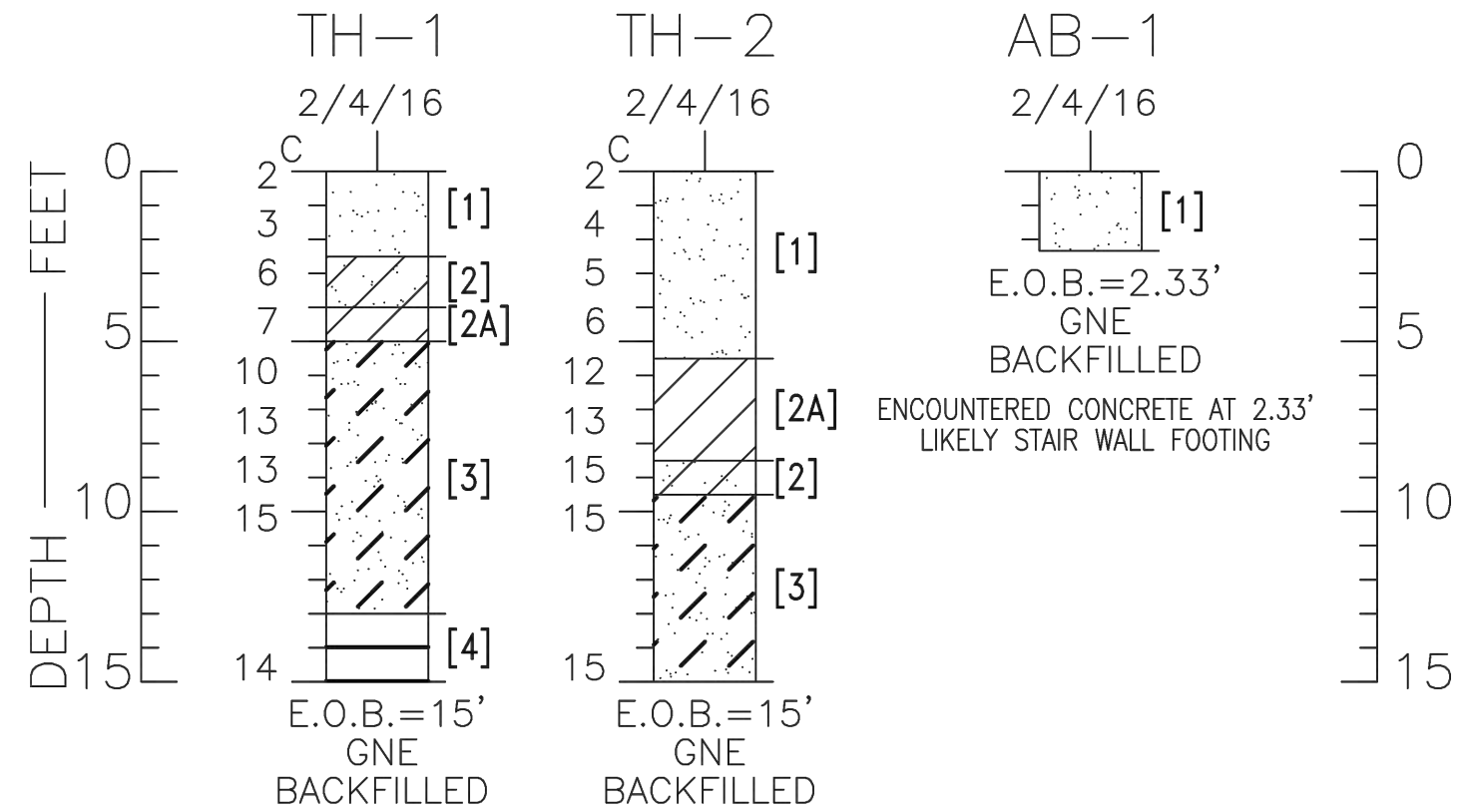
- ⊕ TH/AB APPROXIMATE TEST BORING LOCATION
- \*C DYNAMIC CONE PENETRATION (DCP) "BLOW COUNTS" (ASTM STP#399)
- EOB END OF BORING
- GNE GROUNDWATER NOT ENCOUNTERED
- SP-SM, SM, SC UNIFIED SOIL CLASSIFICATION SYSTEM
- A-3, A-2-4 AASHTO SOIL CLASSIFICATION SYSTEM
- DRILLERS: DA, SH
- DRILL METHOD: HAND AUGERS (ASTM D1452)

\*SEE APPENDIX A FOR CORRELATIONS BETWEEN DCP "C"-VALUES AND SPT "N"-VALUES

TEST BORING LOCATION PLAN  
(GOOGLE EARTH IMAGE, NOT-TO-SCALE)



**SOIL BORING PROFILES**



**SOIL LEGEND**

- [1] LIGHT BROWN TO BROWN SILTY FINE SAND W/CONSTRUCTION DEBRIS, PRIOR FILL (SM; A-2-4)
- [2] ORANGISH-BROWN CLAYEY FINE SAND (SC; A-2-6)
- [2A] REDDISH-BROWN VERY CLAYEY FINE SAND TO VERY SANDY LEAN CLAY (SC TO CL; A-6)
- [3] REDDISH-BROWN TO ORANGISH-BROWN SLIGHTLY CLAYEY TO CLAYEY MEDIUM TO FINE SAND (SC; A-2-4)
- [4] MARBLED LIGHT GRAY & LIGHT BROWN SANDY FAT CLAY (CH; A-7)

WHILE THE BORINGS ARE REPRESENTATIVE OF SUBSURFACE CONDITIONS AT THEIR RESPECTIVE LOCATIONS AND FOR THEIR RESPECTIVE VERTICAL REACHES, LOCAL VARIATIONS CHARACTERISTICS OF THE SUBSURFACE MATERIALS OF THE REGION ARE ANTICIPATED AND MAY BE ENCOUNTERED. THE BORING LOGS AND RELATED INFORMATION ARE BASED ON THE DRILLER'S LOGS AND VISUAL EXAMINATION OF SELECTED SAMPLES IN THE LABORATORY. THE DELINEATION BETWEEN SOIL TYPES SHOWN ON THE LOGS IS APPROXIMATE AND THE DESCRIPTION REPRESENTS OUR INTERPRETATION OF SUBSURFACE CONDITIONS AT THE DESIGNATED BORING LOCATIONS ON THE PARTICULAR DATE DRILLED. GROUNDWATER ELEVATIONS SHOWN ON THE BORING LOGS REPRESENT GROUNDWATER SURFACES ENCOUNTERED ON THE DATES SHOWN. FLUCTUATIONS IN WATER TABLE LEVELS SHOULD BE ANTICIPATED THROUGHOUT THE YEAR. ABSENCE OF WATER SURFACE DATA ON CERTAIN BORINGS IMPLIES THAT NO GROUNDWATER DATA IS AVAILABLE, BUT DOES NOT NECESSARILY MEAN THAT GROUNDWATER WILL NOT BE ENCOUNTERED AT THESE LOCATIONS OR WITHIN THE VERTICAL REACHES OF THESE BORINGS IN THE FUTURE.

**Ardaman & Associates, Inc.**  
3175 W. Tharpe Street  
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SHEET TITLE:  
SUBSURFACE SOIL EXPLORATION AT THE  
DCF, 2383 PHILLIPS ROAD  
NORTHEAST STAIRWELL MOVEMENT  
TALLAHASSEE, LEON COUNTY, FLORIDA

DRAWN BY: JEREMY CLARK | CHECKED BY: MICHAEL WILSON | DATE: FEBRUARY 24, 2016  
FILE NO. 113-16-40-1380 | APPROVED BY: M.S. WILSON, P.E. | FIGURE 1

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SOIL BORINGS

S0.3

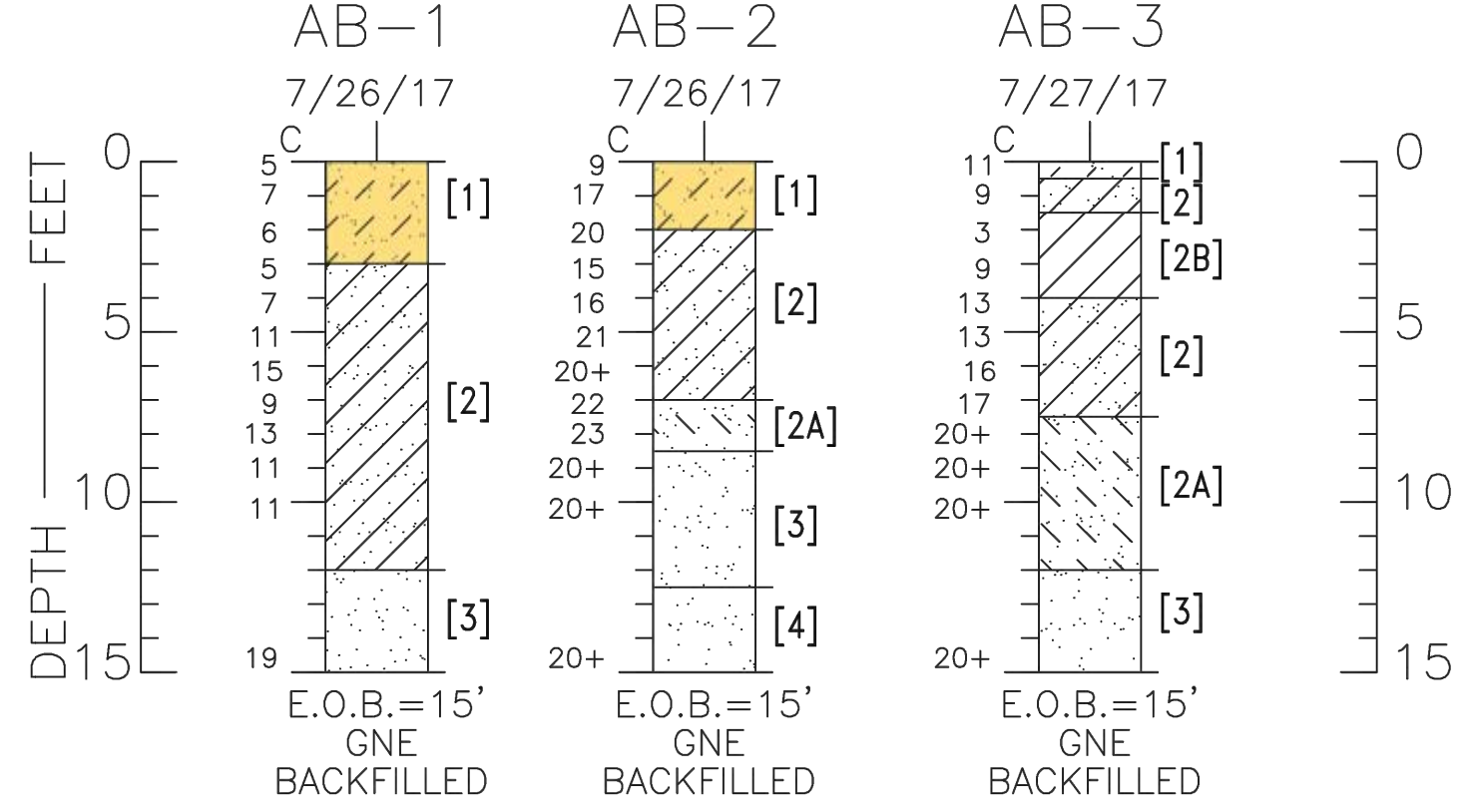
**FIGURE LEGEND**

- ⊕ AB APPROXIMATE TEST BORING LOCATION
- \*C DYNAMIC CONE PENETRATION (DCP) "BLOW COUNTS" (ASTM STP#399)
- EOB END OF BORING
- GNE GROUNDWATER NOT ENCOUNTERED
- █ POSSIBLE FILL MATERIAL
- SP-SM, SM, SC UNIFIED SOIL CLASSIFICATION SYSTEM
- A-3, A-2-4 AASHTO SOIL CLASSIFICATION SYSTEM
- DRILLERS: KM, SH, DK
- DRILL METHOD: HAND AUGERS (ASTM D1452)

TEST BORING LOCATION PLAN  
(GOOGLE EARTH IMAGE, NOT-TO-SCALE)



**SOIL BORING PROFILES**



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**SOIL LEGEND**

- [1] LIGHT BROWN TO BROWN SILTY FINE SAND W/CLAY (SM; A-2-4)
- [2] LIGHT ORANGISH-BROWN TO ORANGISH-BROWN CLAYEY FINE SAND (SC; A-2-6)
- [2A] LIGHT ORANGISH-BROWN SLIGHTLY CLAYEY, SILTY MEDIUM TO FINE SAND (SM; A-2-4)
- [2B] ORANGISH-BROWN VERY CLAYEY FINE SAND TO VERY SANDY LEAN CLAY (SC TO CL; A-6)
- [3] YELLOWISH TAN MEDIUM TO FINE SAND SOMETIMES WITH SILT (SP-SM; A-3)

**ENGINEERING CLASSIFICATION**

I COHESIONLESS SOILS		
DESCRIPTION	"N" ≈ "c"	
VERY LOOSE	0 TO 4	
LOOSE	4 TO 10	
MEDIUM DENSE	10 TO 30	
DENSE	30 TO 50	
VERY DENSE	>50	
II COHESIVE SOILS		
DESCRIPTION	UNCONFINED COMPRESSIVE STRENGTH, QU, TSF	"N" ≈ "c"
VERY SOFT	<1/4	0 TO 2
SOFT	1/4 TO 1/2	2 TO 4
MEDIUM STIFF	1/2 TO 1	4 TO 8
STIFF	1 TO 2	8 TO 15
VERY STIFF	2 TO 4	15 TO 30
HARD	>4	>30

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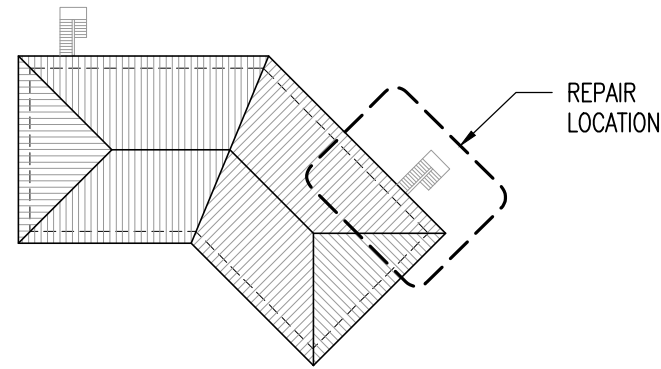
DRAWN BY: ART      CHECKED BY: JMC      DATE: JULY 31, 2017  
FILE NO. 113-17-40-1862      APPROVED BY: J.M. CLARK, P.E.      FIGURE 1

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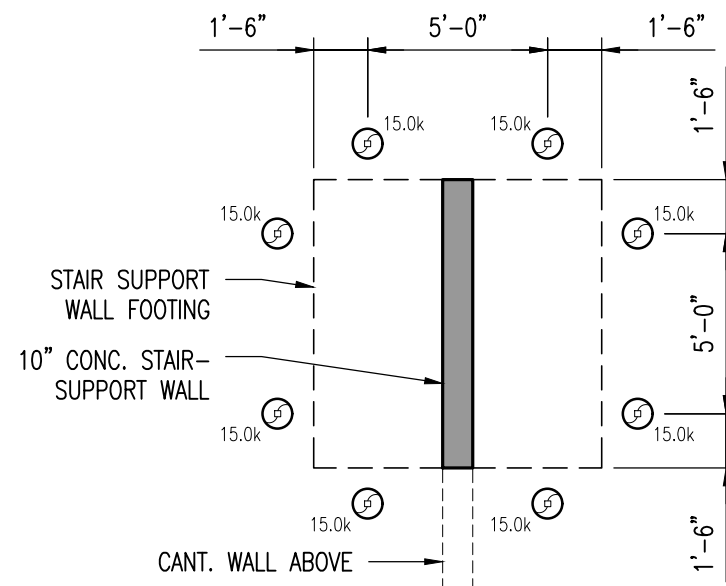
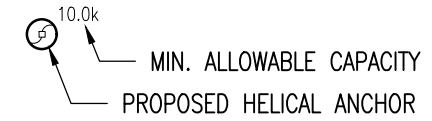
SOIL BORINGS

S0.4

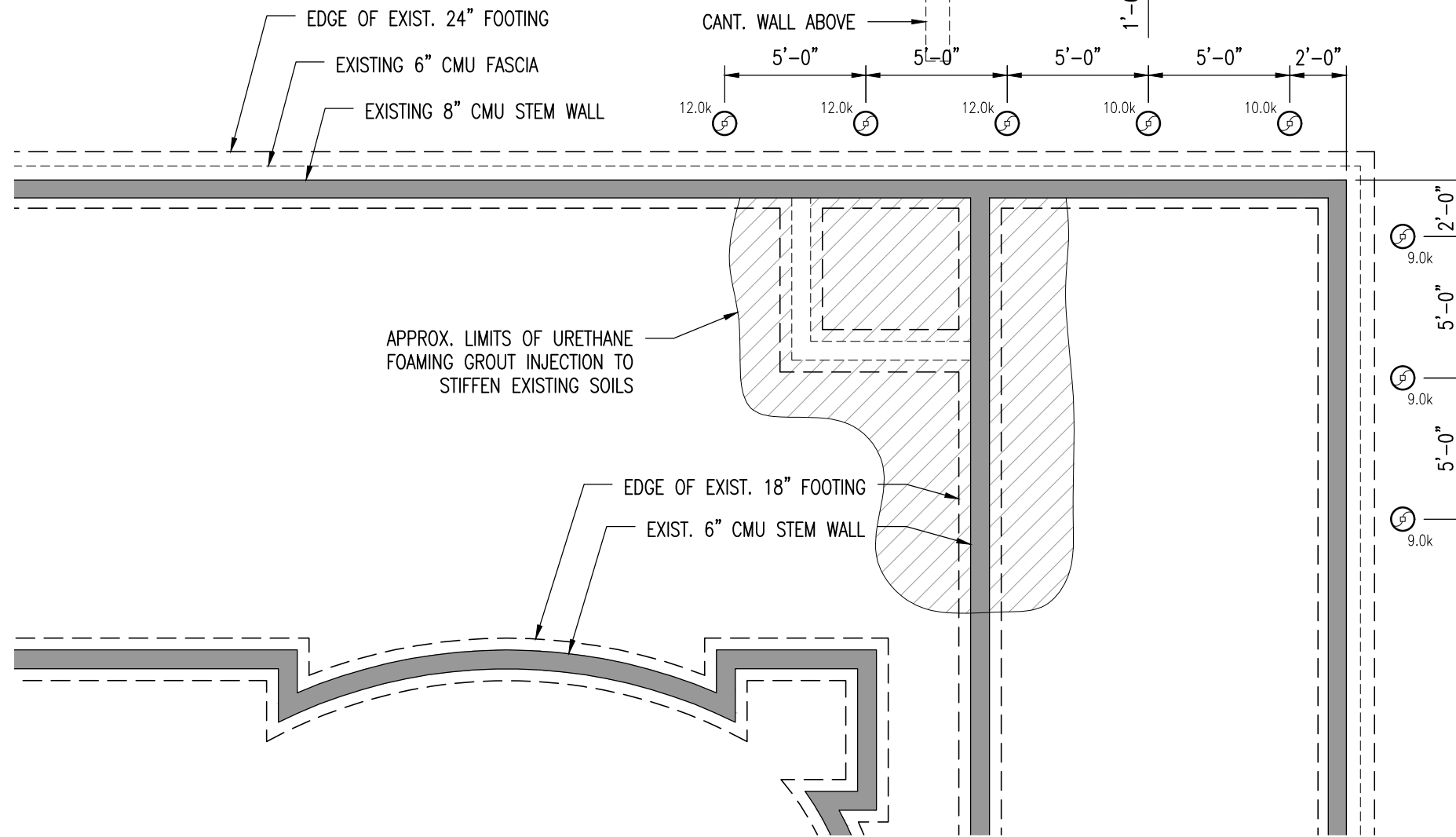


KEY PLAN  
SCALE: 1/64" = 1'-0"

LEGEND



NOTE:  
PROVIDE URETHANE FOAMING GROUT  
BENEATH EXISTING STAIR FOOTING  
AFTER LIFTING HAS BEEN COMPLETED.



HELICAL PILE LAYOUT PLAN  
SCALE: 3/16" = 1'-0"



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HELICAL PILE LAYOUT PLAN

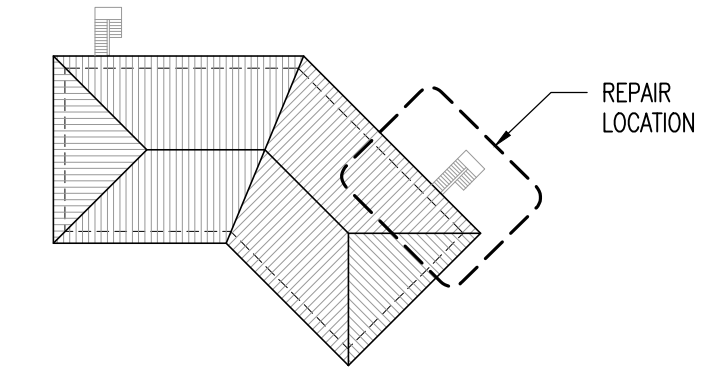
S1.1

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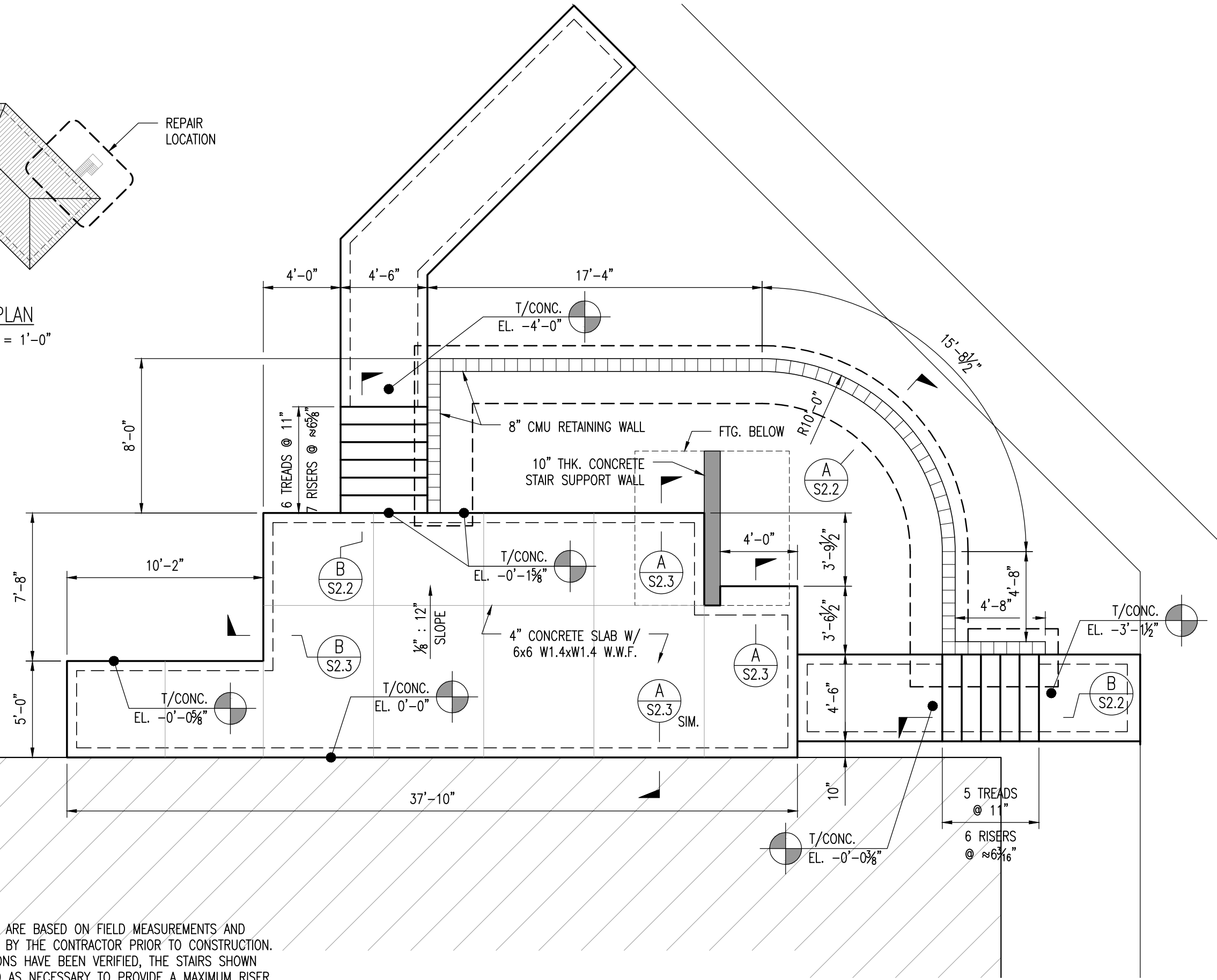
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RETAINING WALL & SIDEWALK PLAN

S1.2



KEY PLAN  
SCALE: 1/64" = 1'-0"



- NOTES:
- ELEVATIONS SHOWN ARE BASED ON FIELD MEASUREMENTS AND SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION.
  - ONCE THE ELEVATIONS HAVE BEEN VERIFIED, THE STAIRS SHOWN SHALL BE MODIFIED AS NECESSARY TO PROVIDE A MAXIMUM RISER HEIGHT OF 7 INCHES.

RET. WALL & SIDEWALK PLAN  
SCALE: 3/16" = 1'-0"

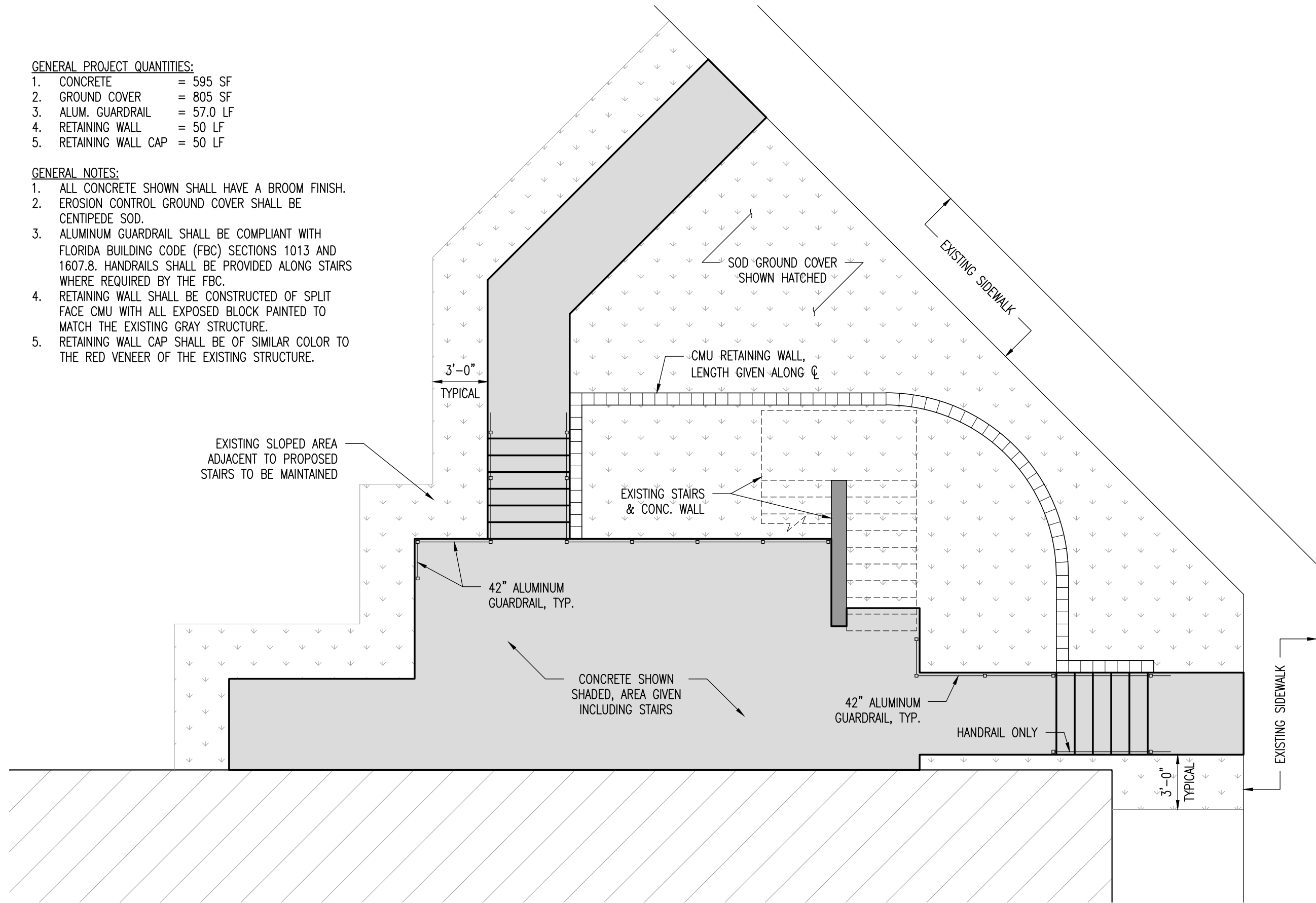


**GENERAL PROJECT QUANTITIES:**

1. CONCRETE = 595 SF
2. GROUND COVER = 805 SF
3. ALUM. GUARDRAIL = 57.0 LF
4. RETAINING WALL = 50 LF
5. RETAINING WALL CAP = 50 LF

**GENERAL NOTES:**

1. ALL CONCRETE SHOWN SHALL HAVE A BROOM FINISH.
2. EROSION CONTROL GROUND COVER SHALL BE CENTIPEDE SOD.
3. ALUMINUM GUARDRAIL SHALL BE COMPLIANT WITH FLORIDA BUILDING CODE (FBC) SECTIONS 1013 AND 1607.8. HANDRAILS SHALL BE PROVIDED ALONG STAIRS WHERE REQUIRED BY THE FBC.
4. RETAINING WALL SHALL BE CONSTRUCTED OF SPLIT FACE CMU WITH ALL EXPOSED BLOCK PAINTED TO MATCH THE EXISTING GRAY STRUCTURE.
5. RETAINING WALL CAP SHALL BE OF SIMILAR COLOR TO THE RED VENEER OF THE EXISTING STRUCTURE.



**GENERAL PROJECT QUANTITIES**

SCALE: 3/16" = 1'-0"

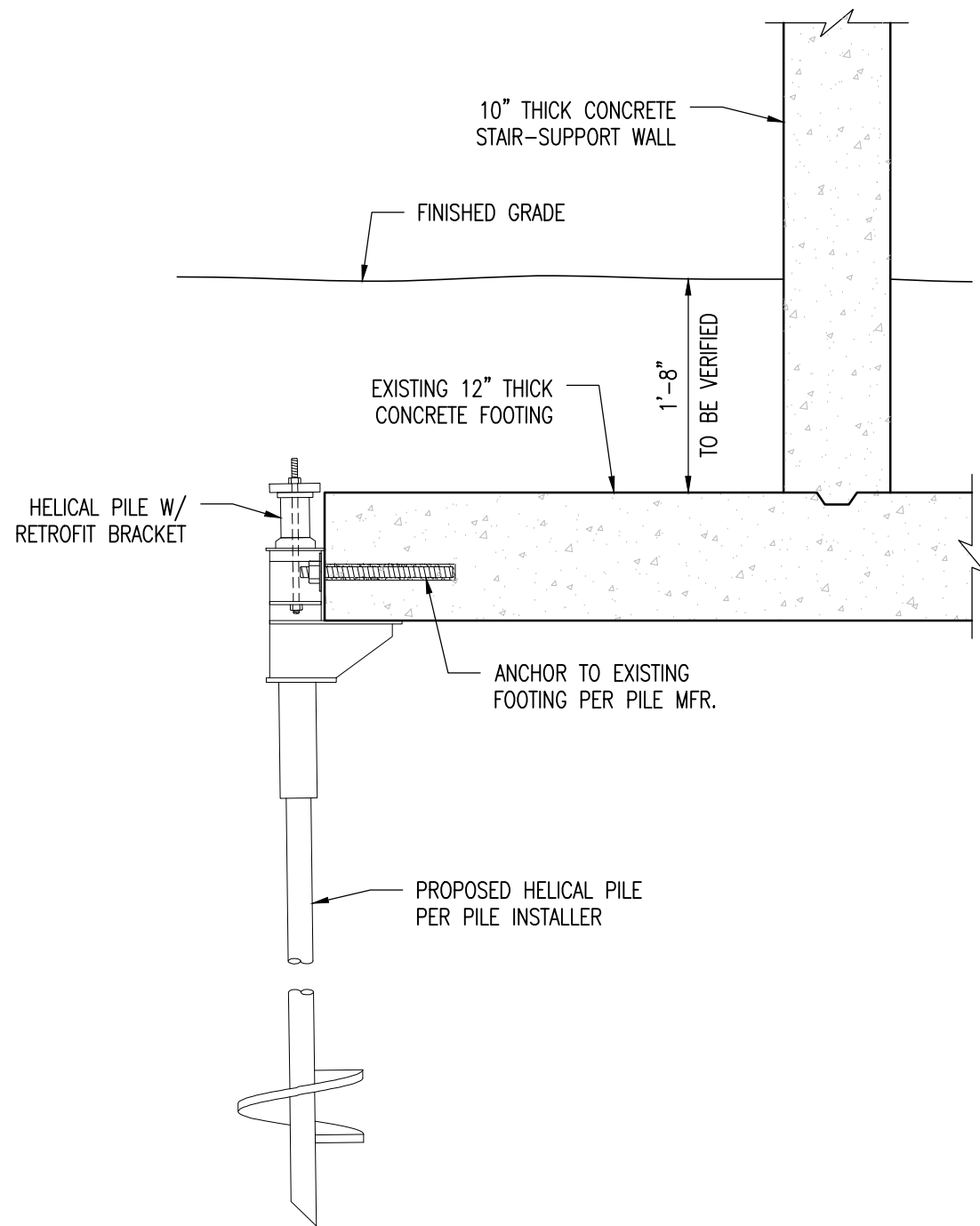


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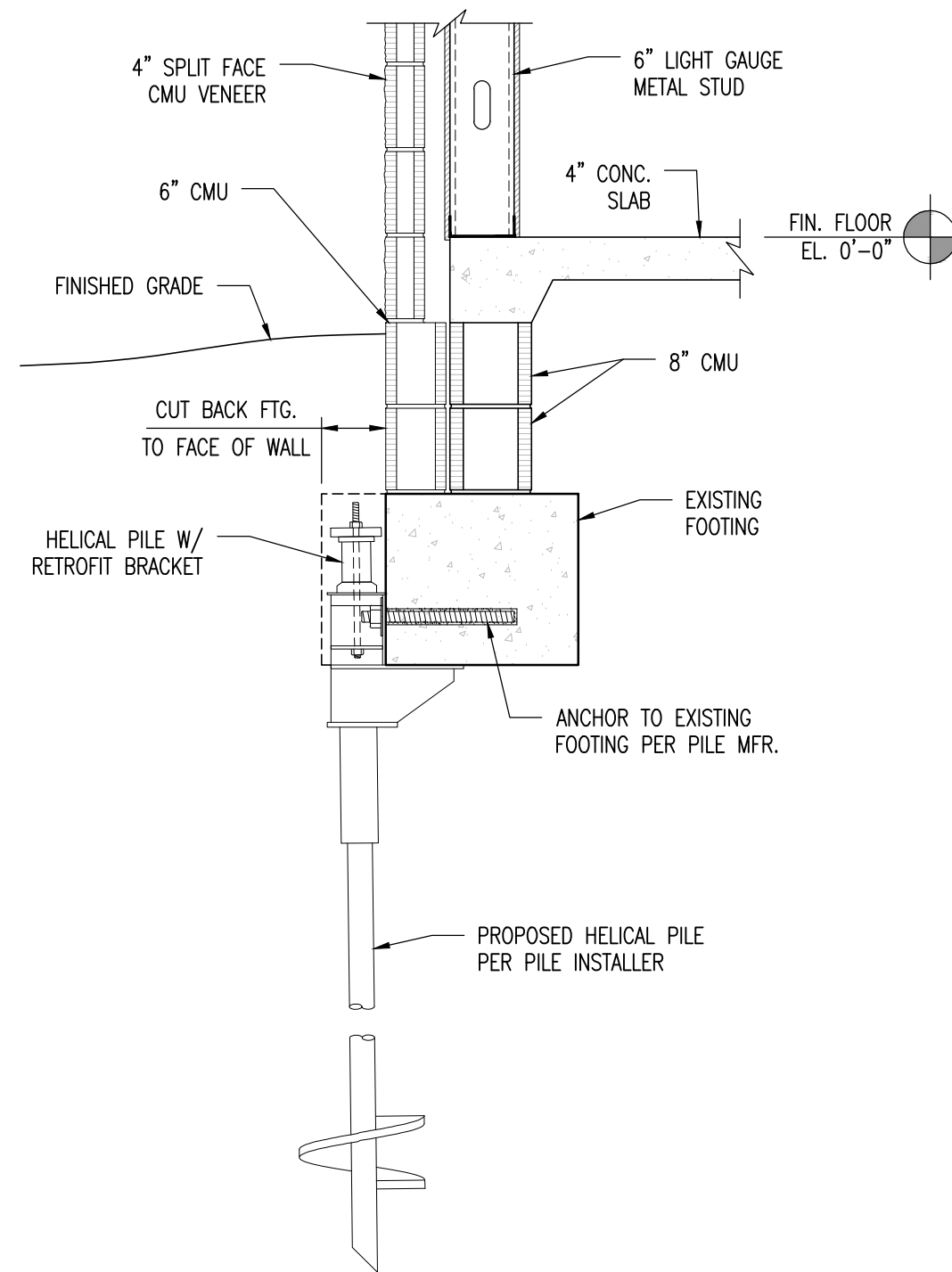
**GENERAL PROJECT QUANTITIES**

**S1.3**



STAIR SUPPORT WALL FTG.  
SCALE: 3/4" = 1'-0"

A  
S2.1



TYP. EXTERIOR WALL FTG.  
SCALE: 3/4" = 1'-0"

B  
S2.1

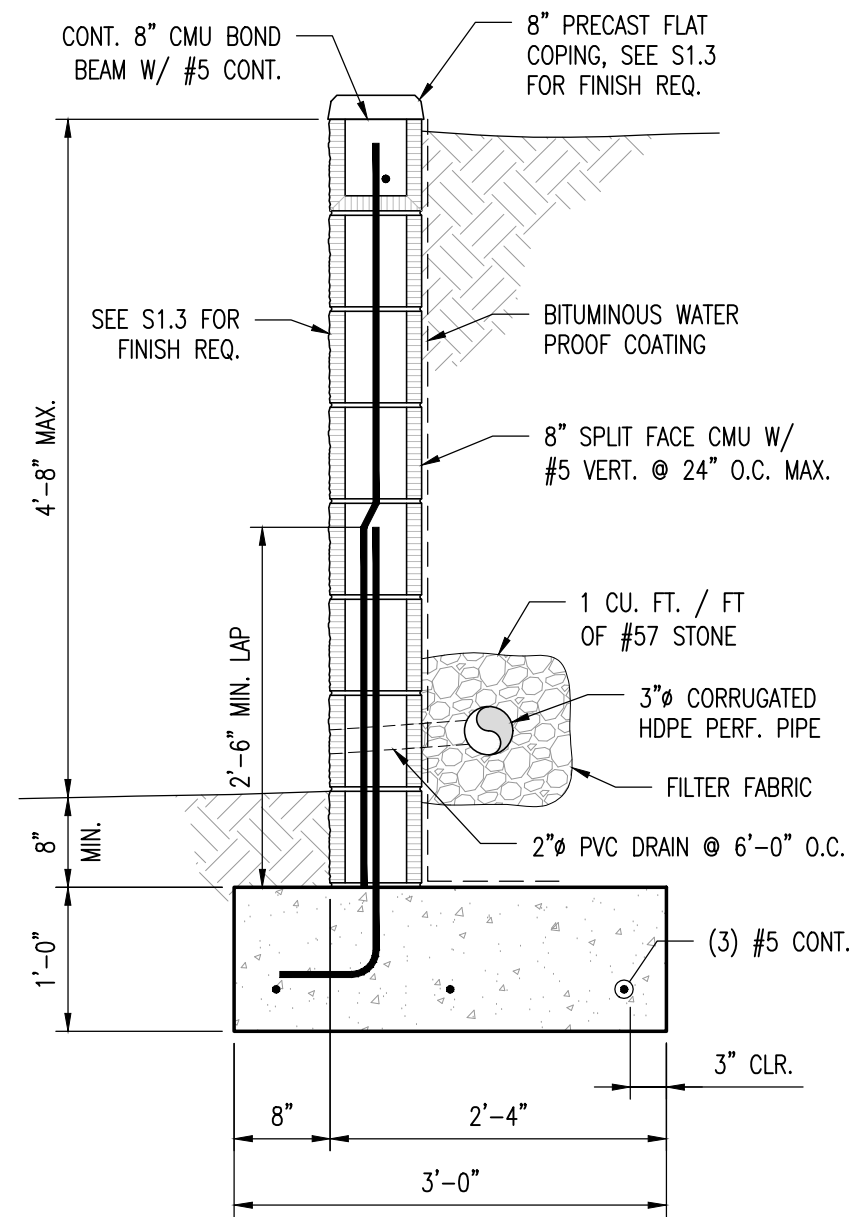
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FOOTING SECTIONS

S2.1

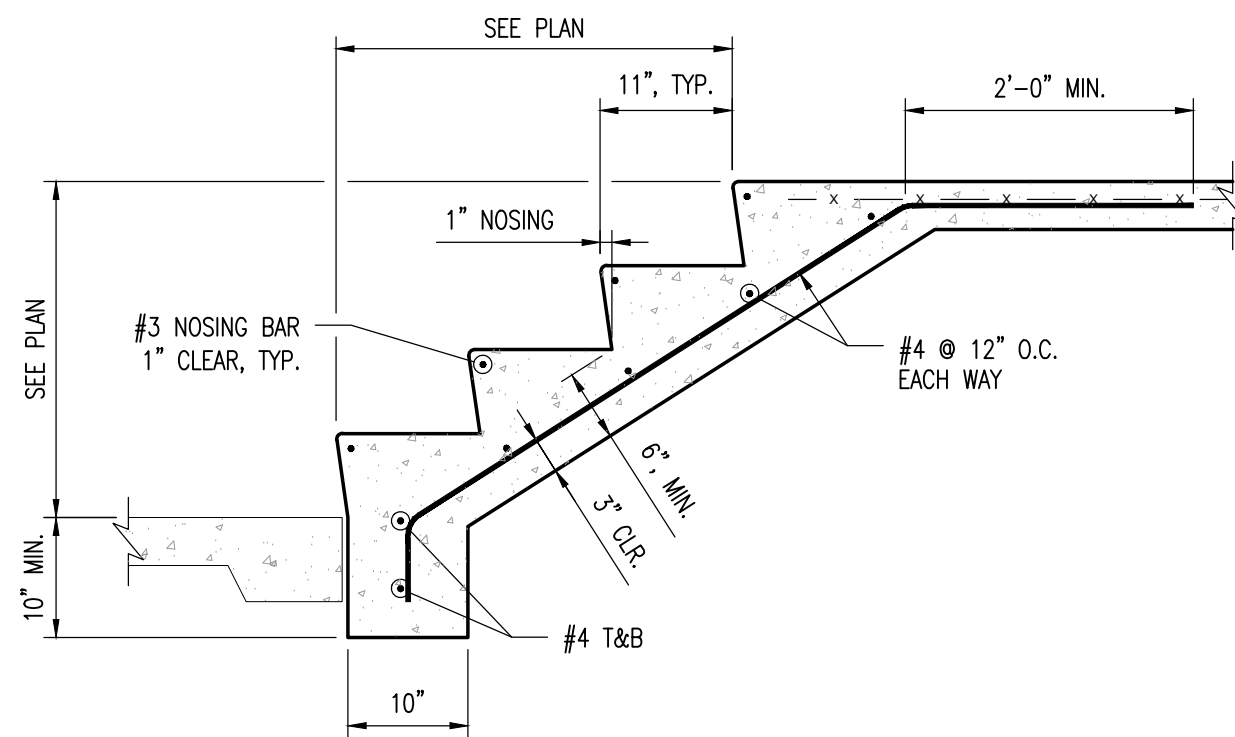




**RETAINING WALL SECTION**

SCALE:  $\frac{3}{4}'' = 1'-0''$

**A**  
S2.2



**TYPICAL STAIR SECTION**

SCALE:  $\frac{3}{4}'' = 1'-0''$

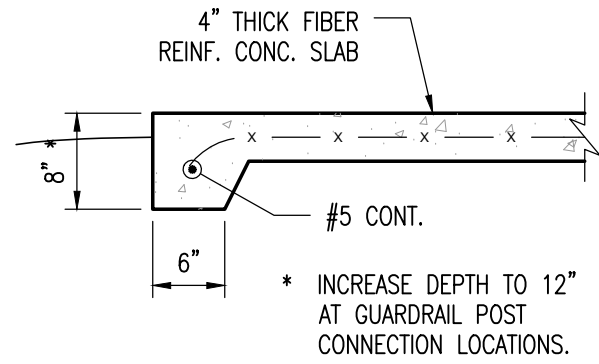
**B**  
S2.2

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RETAINING WALL & STAIR DETAILS

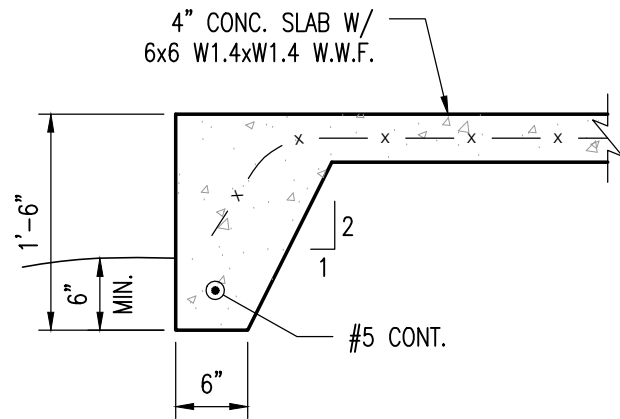
S2.2



**SLAB EDGE DETAIL**

SCALE: 3/4" = 1'-0"

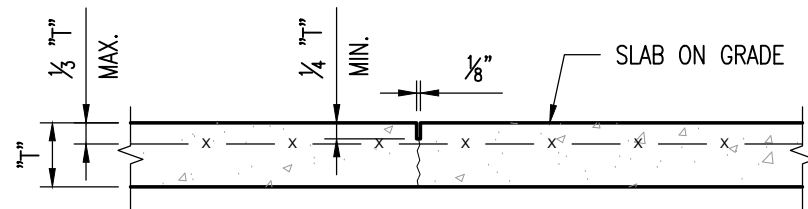
**A**  
S2.3



**SLAB EDGE DETAIL**

SCALE: 3/4" = 1'-0"

**B**  
S2.3

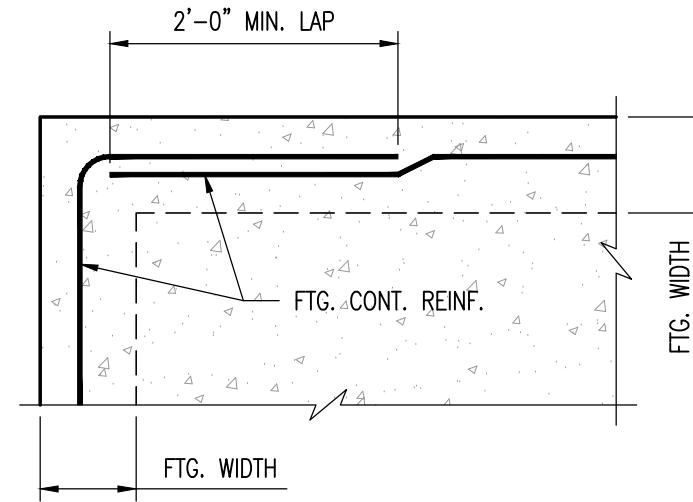


NOTE: SAWCUT SLAB WITHIN 12 HOURS OF POURING SLAB

**TYPICAL SLAB SAWCUT DETAIL**

SCALE: 1" = 1'-0"

**C**  
S2.3



**FOOTING CORNER BAR DETAIL**

SCALE: 3/4" = 1'-0"

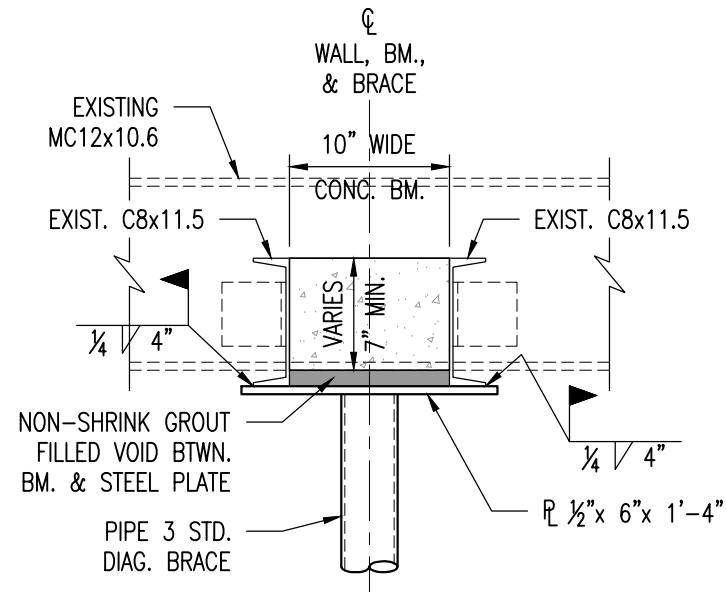
**D**  
S2.3

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SIDEWALK DETAILS

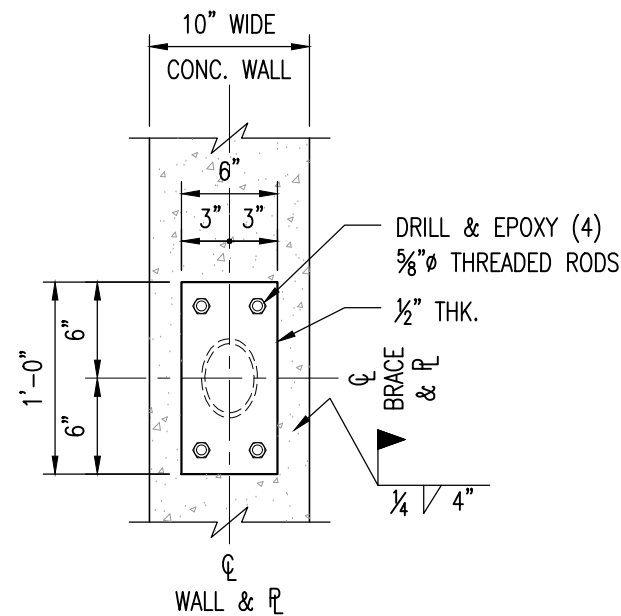
S2.3



**BEAM BEARING PLATE**

SCALE: 1" = 1'-0"

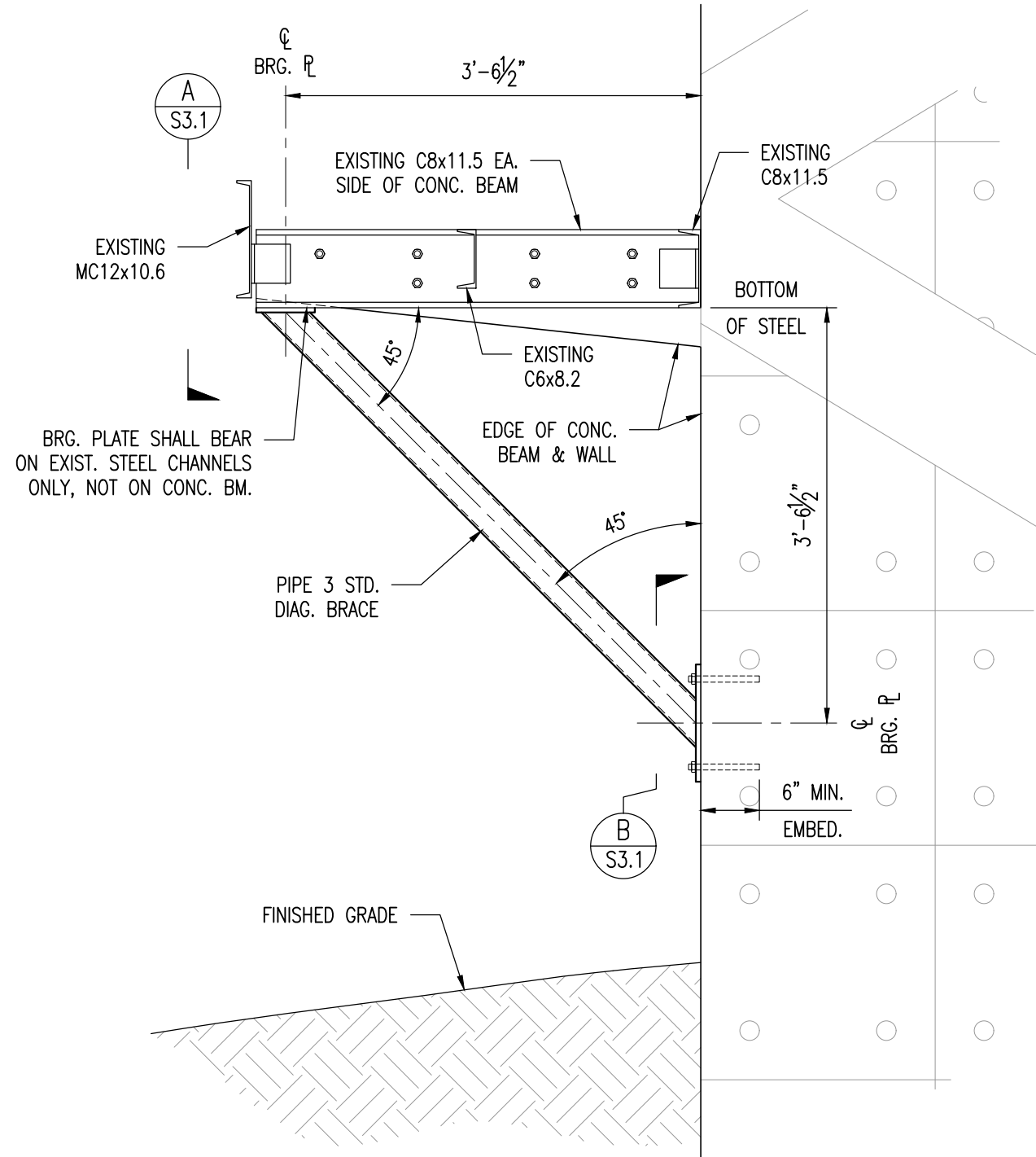
A  
S3.1



**WALL BEARING PLATE**

SCALE: 1" = 1'-0"

B  
S3.1



**STAIR DIAGONAL BRACE**

SCALE: 3/4" = 1'-0"

C  
S3.1

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**STEEL STAIR  
SUPPORT POST**

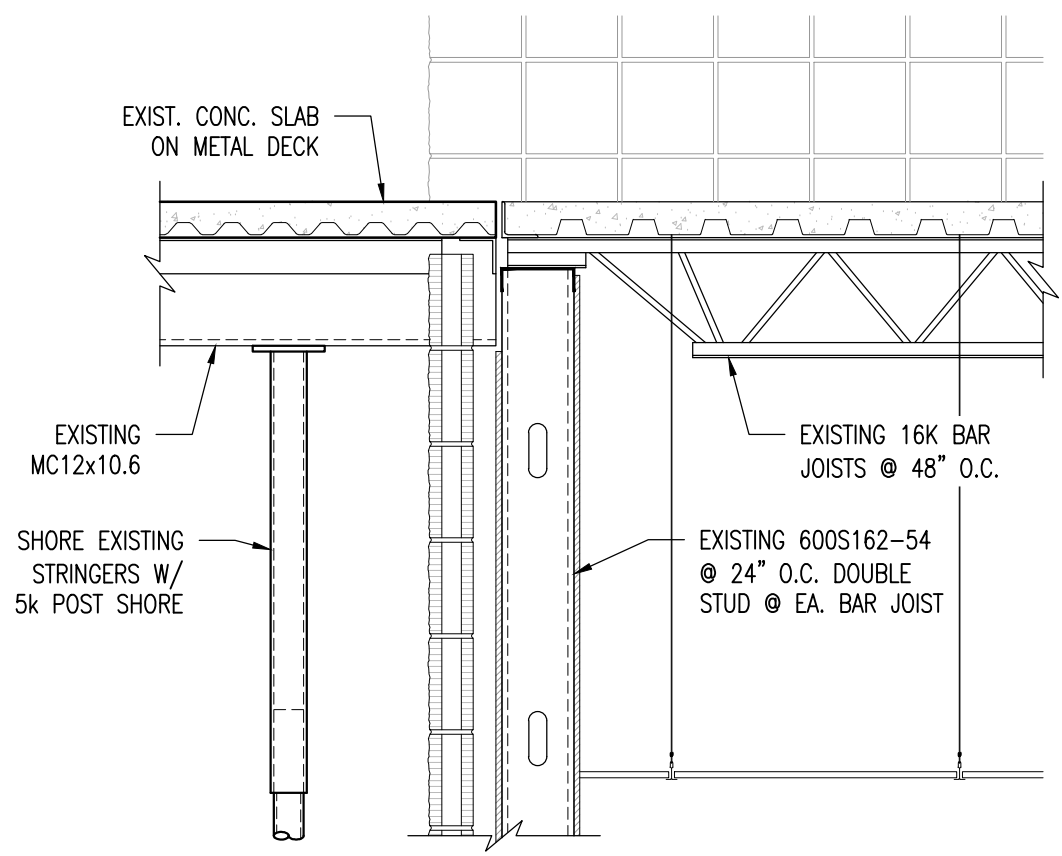
**S3.1**

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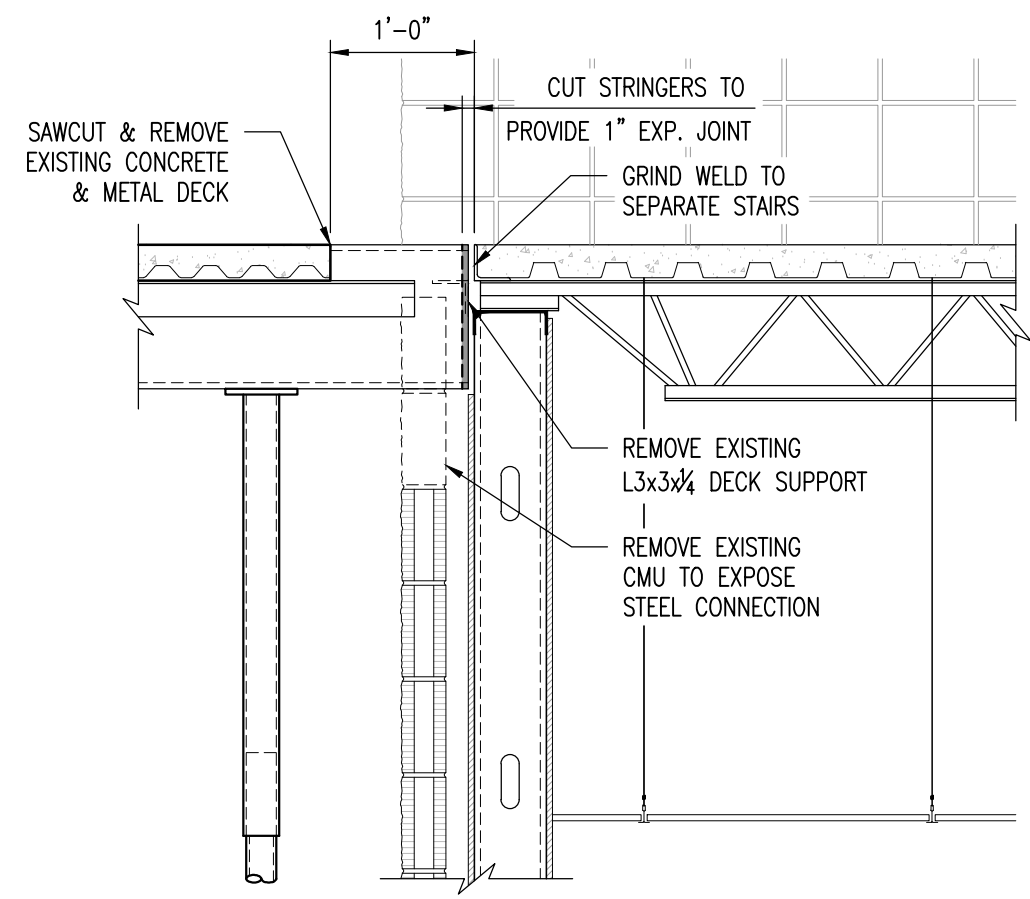
DRAWN	BEK	REV.
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STEEL STAIR EXPANSION JOINT DETAILS

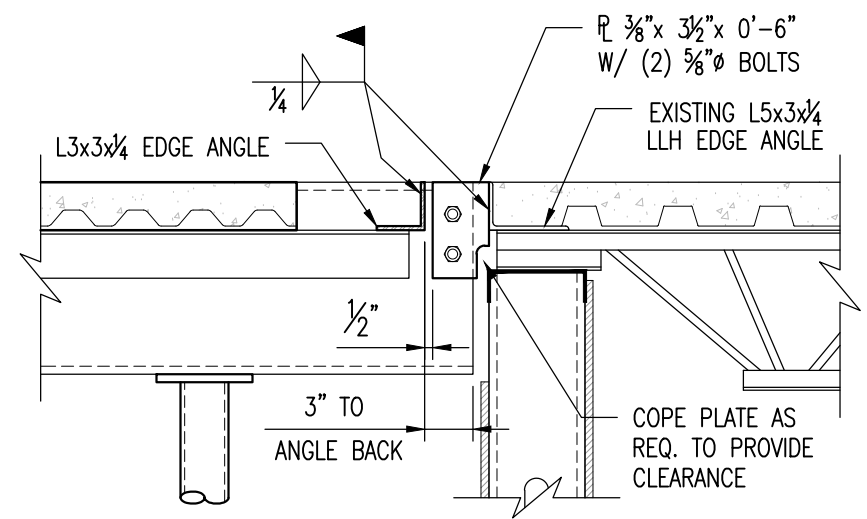
S3.2



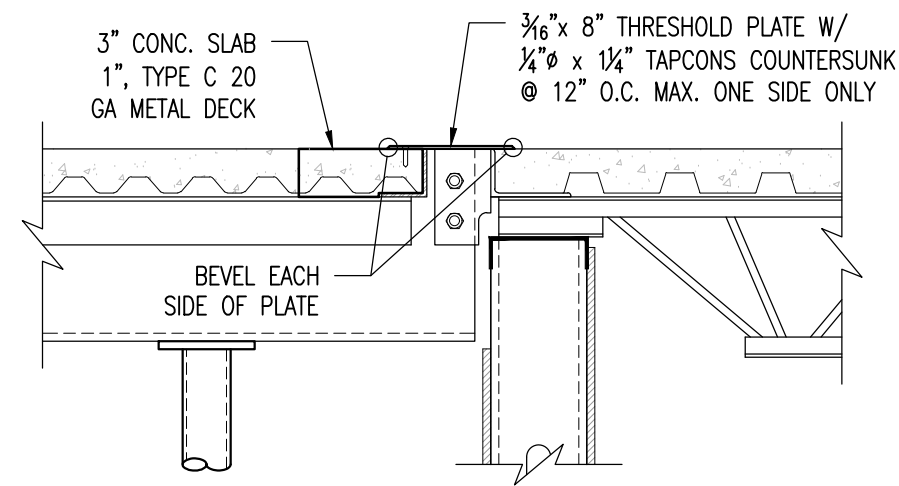
STAIR SUPPORT - PHASE 1 A  
SCALE: 3/4" = 1'-0" S3.2



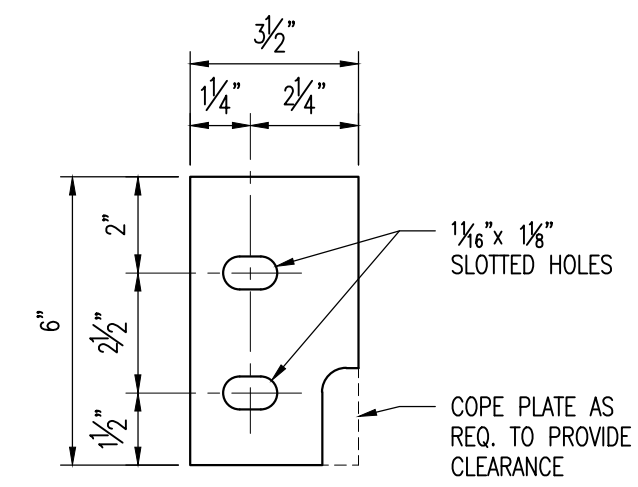
STAIR SUPPORT - PHASE 2 B  
SCALE: 3/4" = 1'-0" S3.2



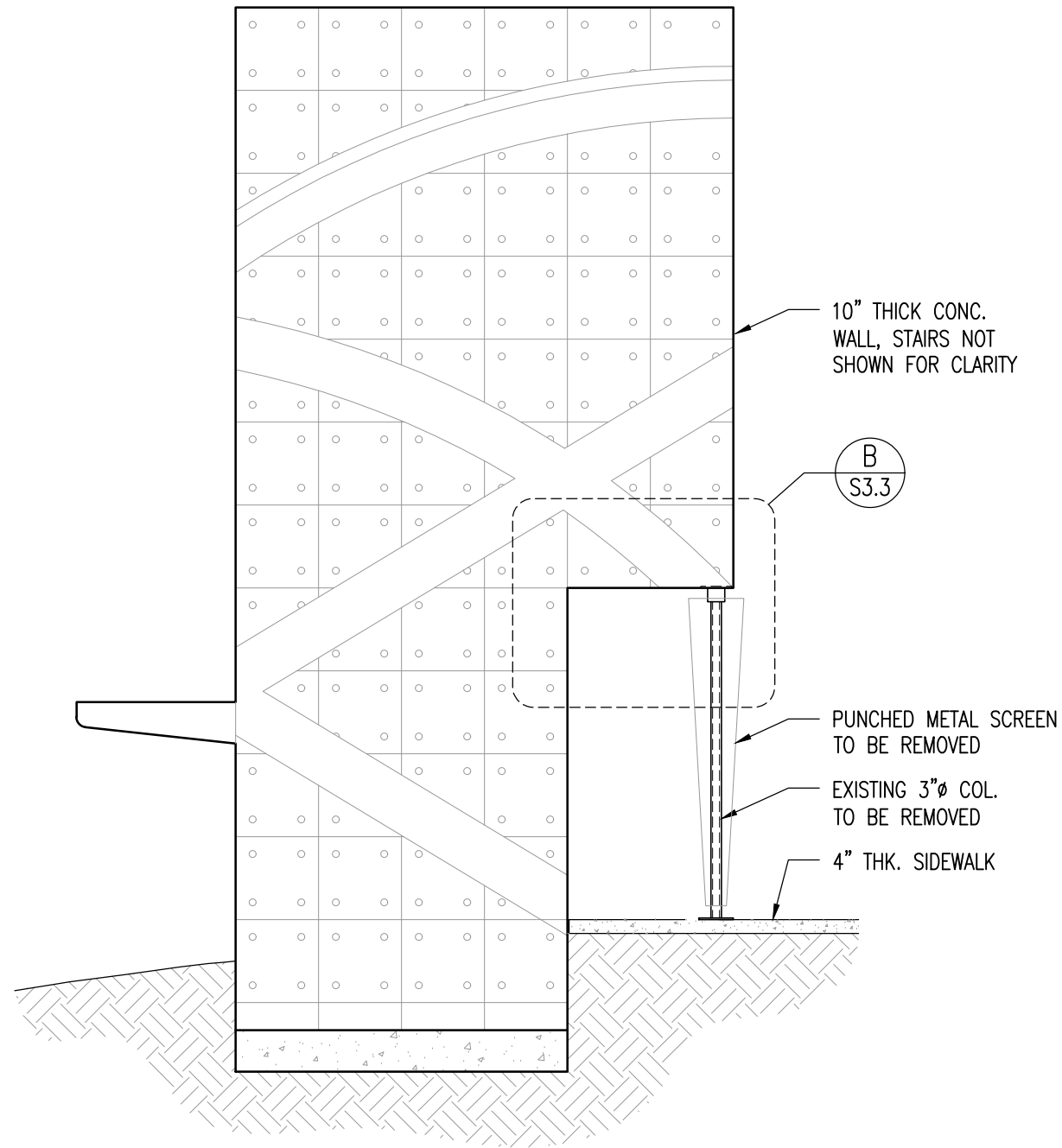
STAIR SUPPORT - PHASE 3 C  
SCALE: 1" = 1'-0" S3.2



STAIR SUPPORT - PHASE 4 D  
SCALE: 1" = 1'-0" S3.2

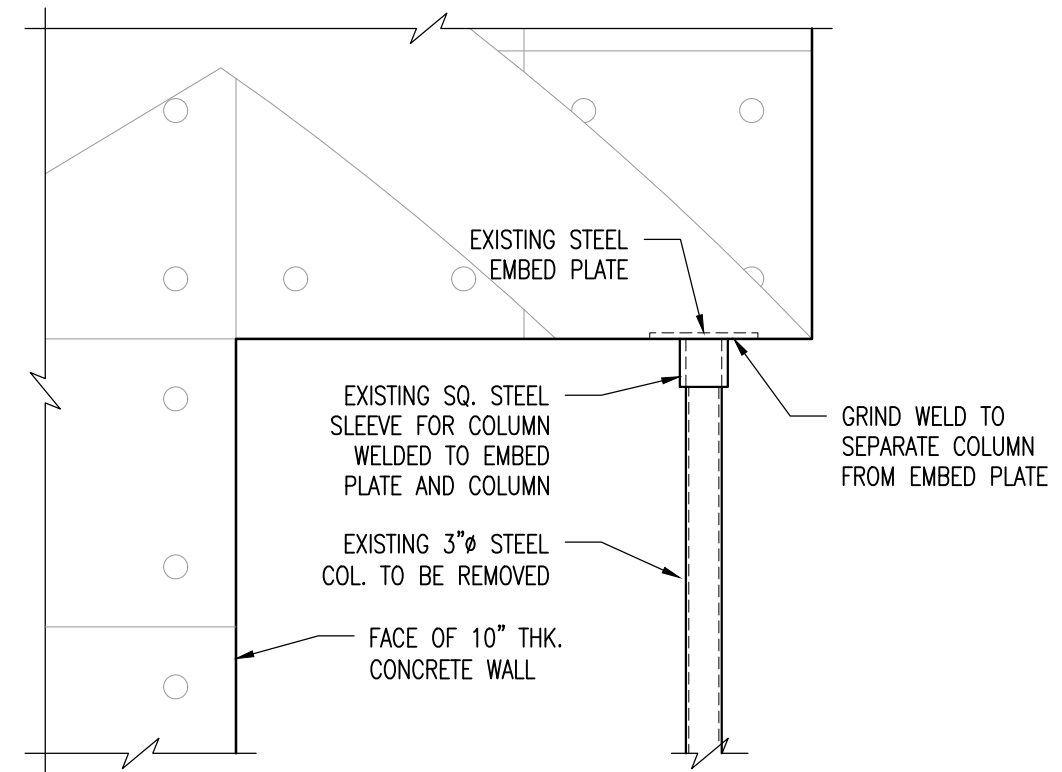


STRINGER CONNECTION PLATE E  
SCALE: 1/2" = 1'-0" S3.2



**EXISTING WALL ELEVATION**  
SCALE: 1/4" = 1'-0"

A  
S3.3



**STAIR SUPPORT - PHASE 1**  
SCALE: 3/4" = 1'-0"

B  
S3.3

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DATE	09.13.2017	REV.

COLUMN MODIFICATIONS

S3.3